

Module - 02

Sewer Appurtenances

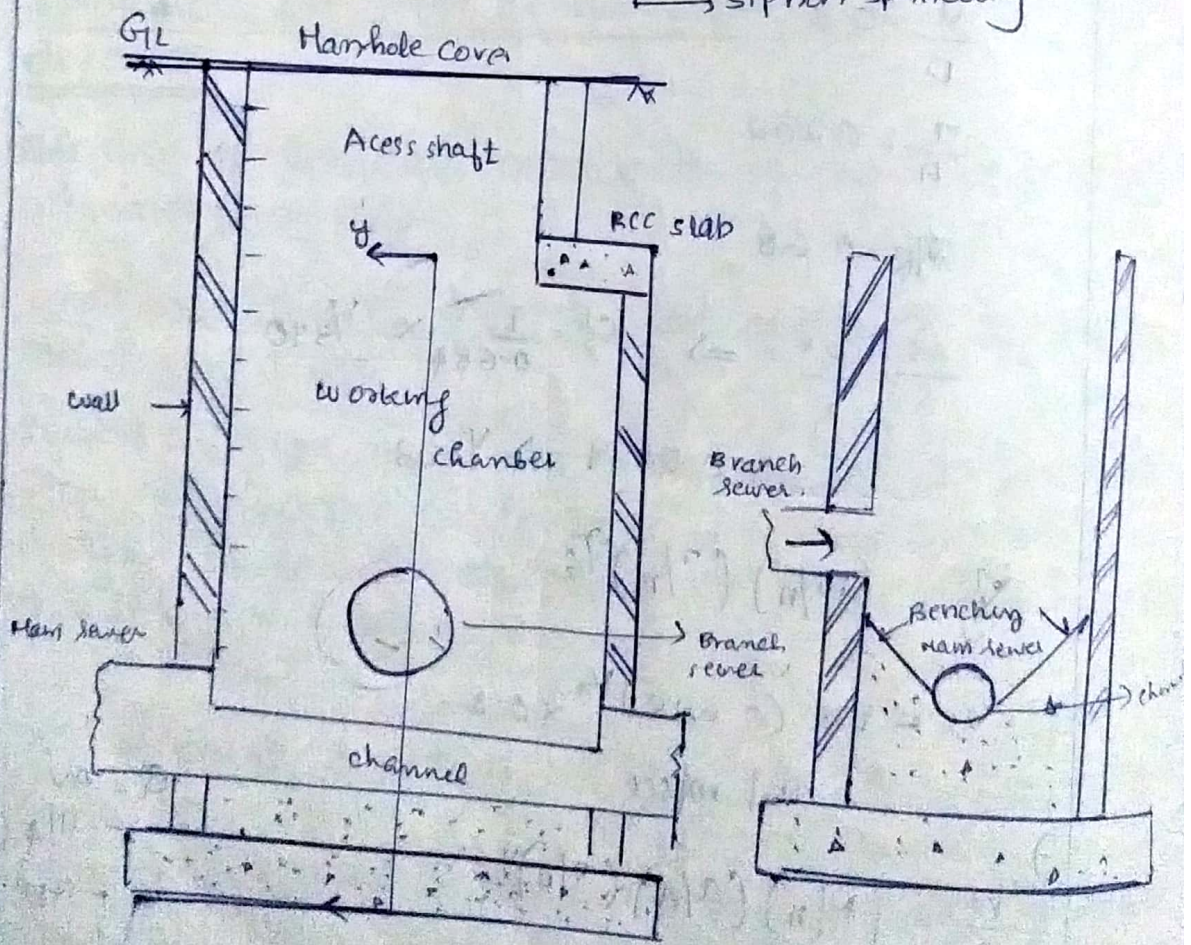
1. Man hole
2. Drop manholes
3. Lamp holes
4. Catch
5. Clean out
6. Flushing tanks
7. Grease and oil trap
8. Inlets

→ shallow
 → Normal
 → Deep

9. Inverted siphon

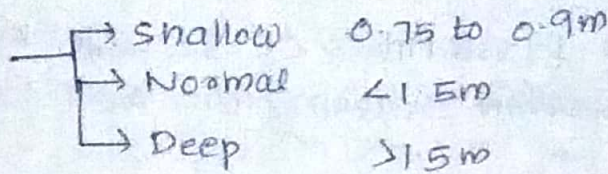
10. Storm regulator

→ leaping weir
 → overflow weir
 → siphon spillway

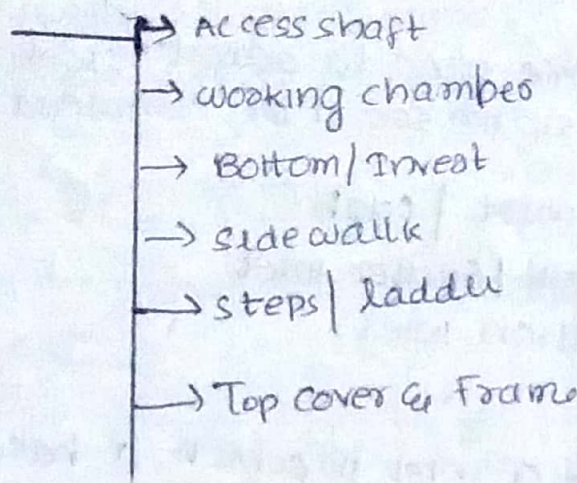


1.
No

Man hole

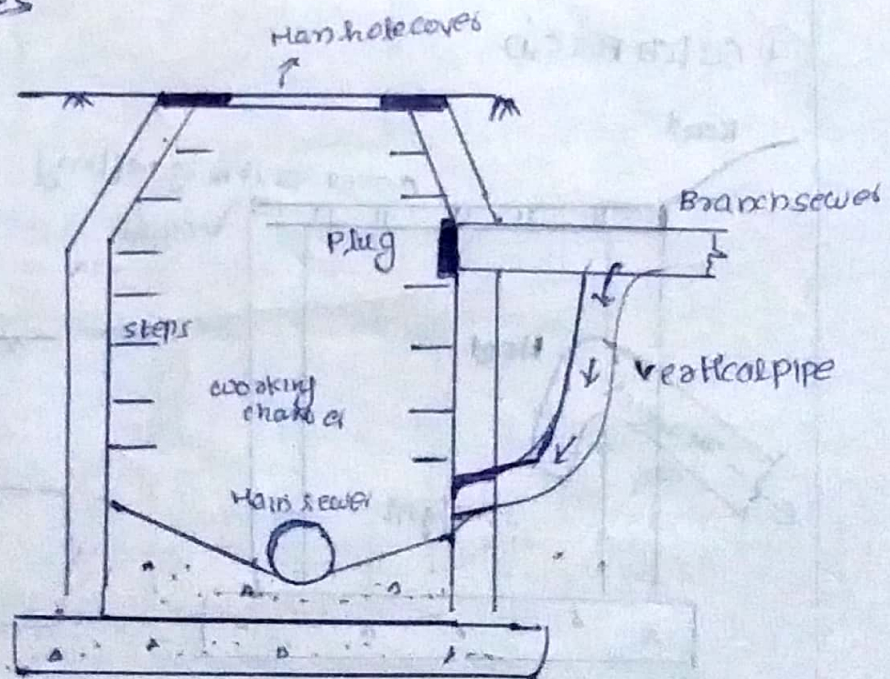


parts



Man hole is a masonry or RCC chamber constructed on the alignment of a sewer for providing access to the sewer for the purpose of inspection testing cleaning etc. These are provided @ every bent, junction change of grade - ent or change of diameters

2. Drop Manholes



A drop manhole is constructed to provide a connection b/w a high level branch sewer to low level main sewer. When a branch sewer enters a man hole while more than 0.5 - 0.6 m above the main sewer the sewage is not allowed to fall directly in to man-hole.

to avoid possibilities of sewage being thrown on persons ~~entering~~ entering the working chamber of the hole

3 **Lamb hole:** provided to

4 **Inlet**

Inlet is a device used to admit storm water and convey it into storm sewer or combined sewer

- vertical inlet / curb
- horizontal / gutter inlet
- combination inlet

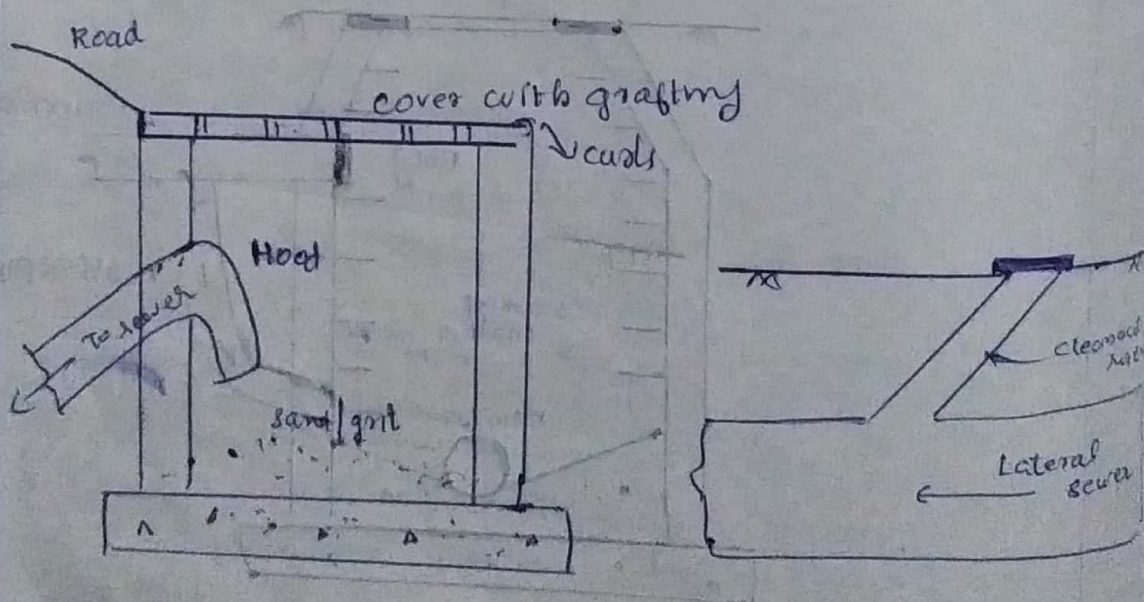
5 **Catch Basins**

This is special type of inlet in which a basin is provided which allows sand, grit, debris

6 **Clean-out**

These are inclined pipes used for clearing out the lateral sewer ^(cleaning)

7 **Catch Basin**



Hood → prevent foul gas

They are street inlet with additional settling basin sand, grit, debris get settled thus prevent their entry to sewer.

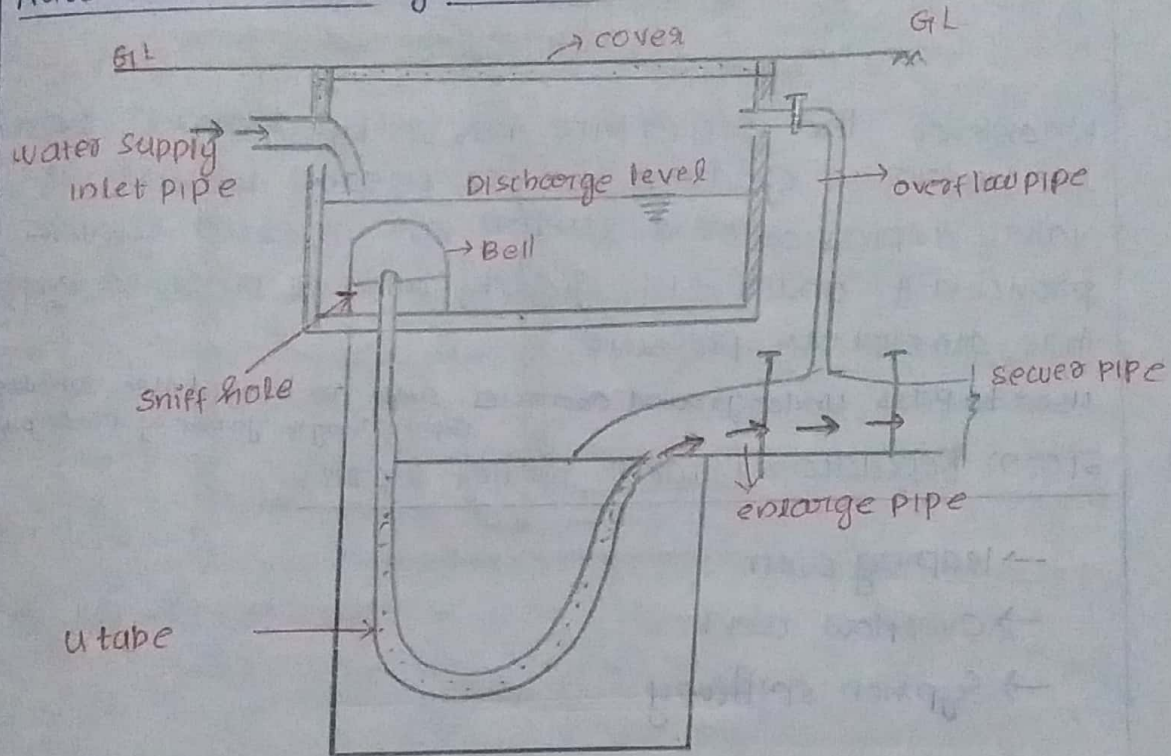
Hood → prevent the escape of foul gases

⑧ Flushing Tank

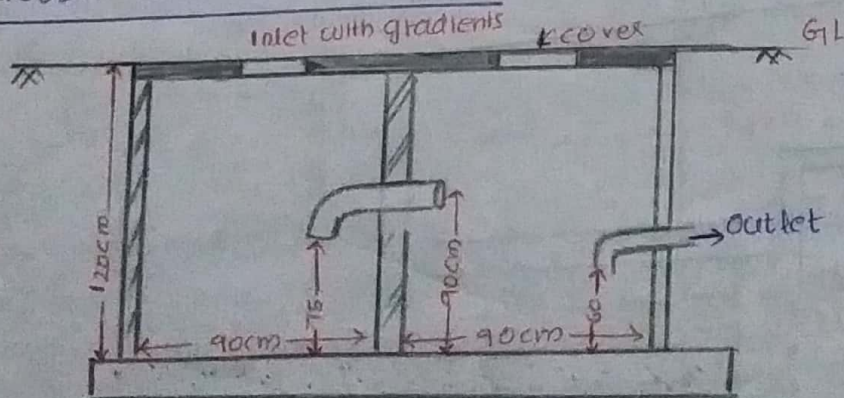
These are used when there are chances of sewer blockage such as in the case of flat gradient not providing self cleansing velocity. The flushing tanks are of 2 types

- (i) Automatic flushing tank
- (ii) Handoperating flushing tank

Automatic flushing tank

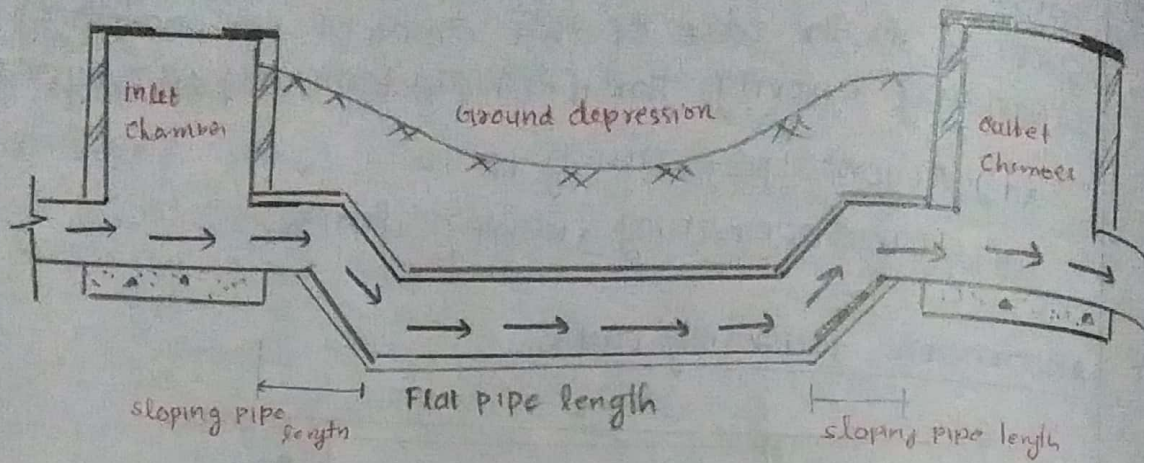


⑨ Grease and oil traps



It is used to remove grease & oil from sewage before it enters into sewerline & these are located @ places such as automobile repair workshops, garages, kitchens of hotels, oil and grease industries etc. If it is not provided the grease and oil will stick onto the sewer pipe material & reduce sewer capacity

10 Inverted Siphons



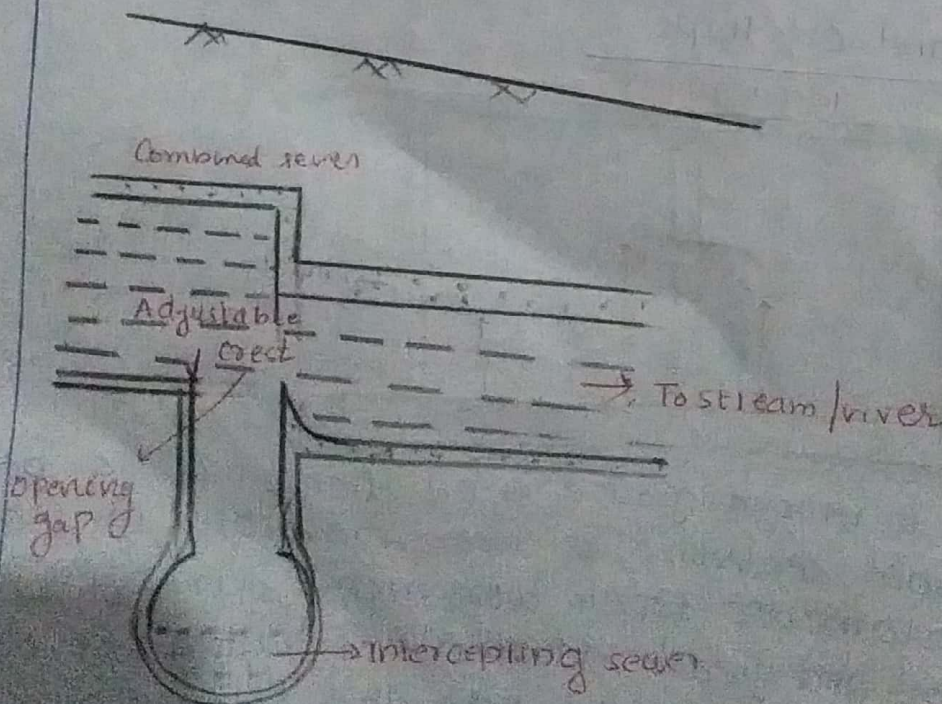
Whenever the sewer pipe has to be dropped below the hydraulic gradient line for passing beneath a valley depression or a stream an inverted siphon is provided it under full gravity with a pressure greater than atmospheric pressure

used to pass underground obstacles such as buried pipe, subway, storm regulators / storm relief works

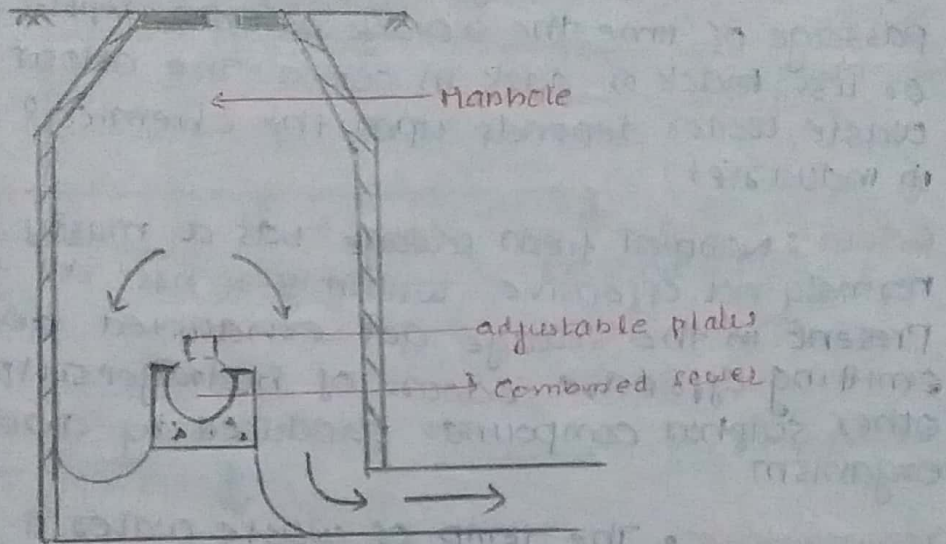
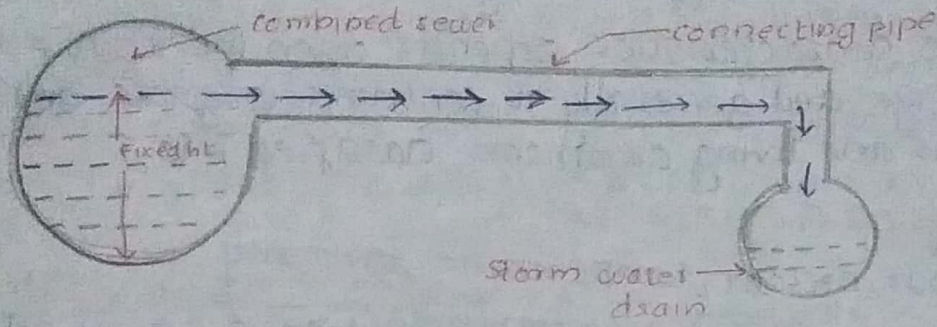
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- leaping weir
- overflow weir
- Siphon spillway

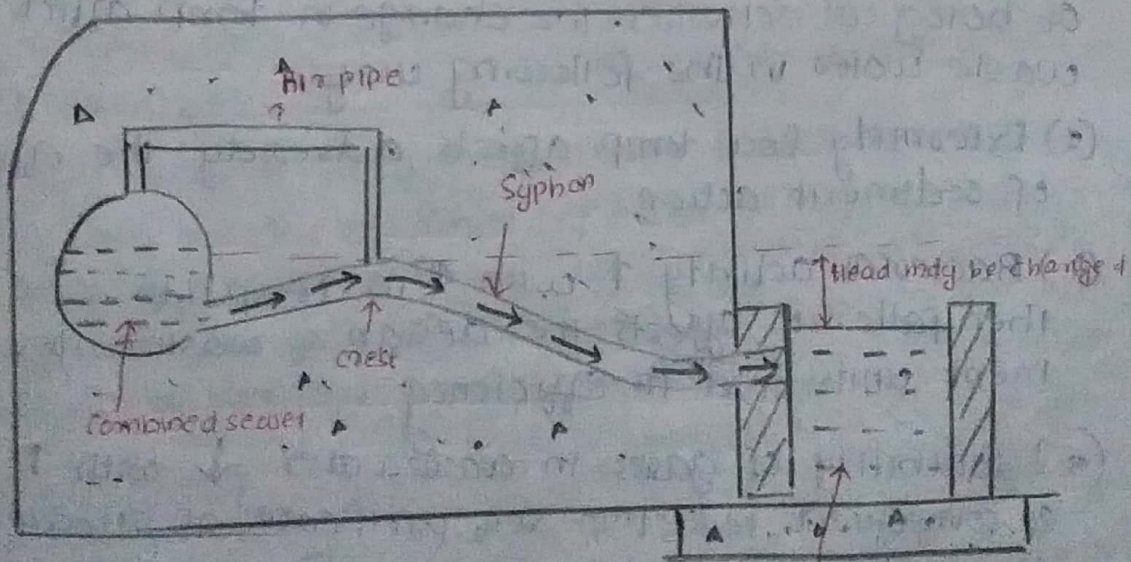
Leaping weir



Overflow weir



Syphon spillway



Storm water drain as
overflow channel

Characteristics of sewage

sewage characteristics depends upon the source of its discharge and generally it contains organic and inorganic matters and living organisms. classified into 3

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1. Physical characteristics :

colour, odour, temperature, turbidity
g/y, light brown → fresh
Black/dark brown → septic or stale

colour: Fresh domestic sewage has a grey colour somewhat resembling a weak solution of soap with the passage of time the sewage become septic and is more or less black or dark in colour. The colour of industrial waste water depends upon the chemical process used in industries

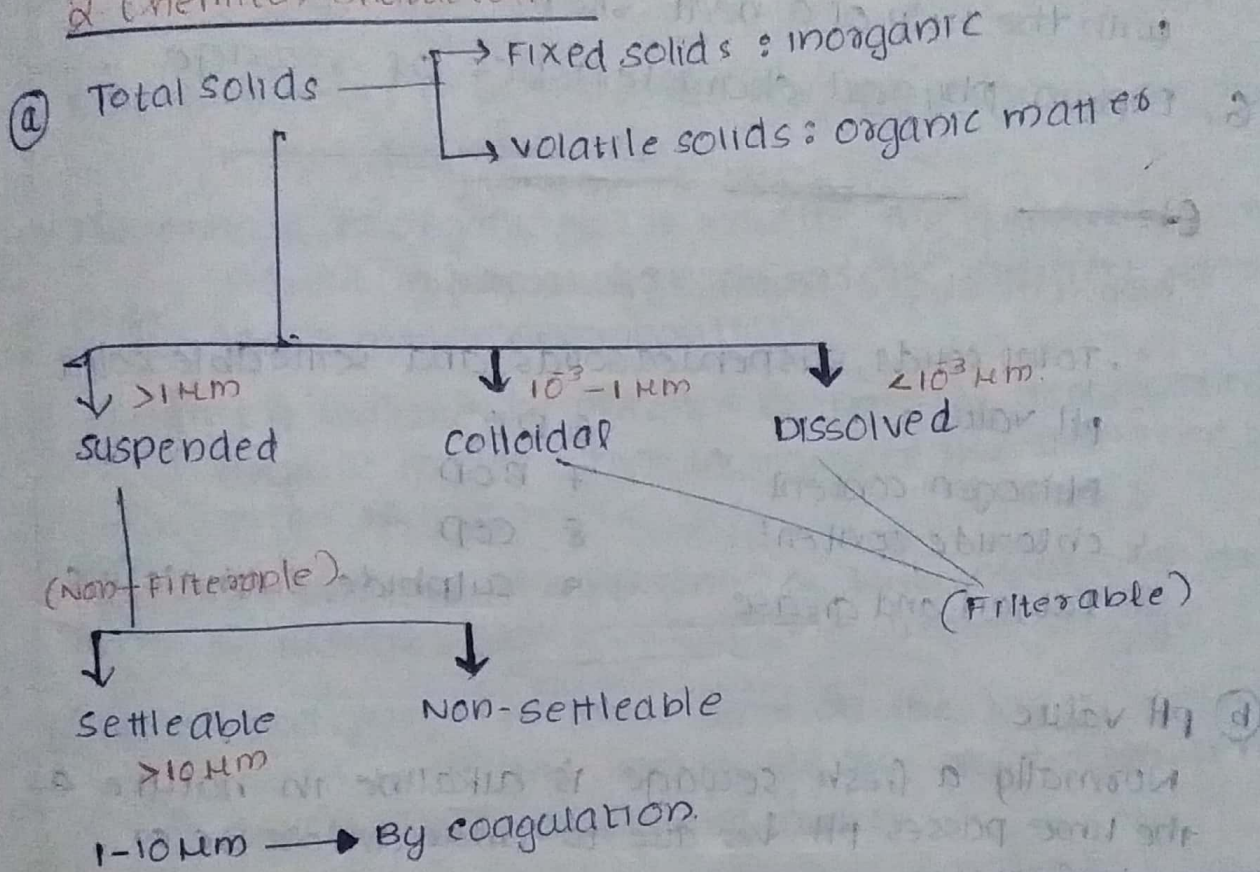
Odour: Normal fresh sewage has a musty odour which is normally not offensive. Within 3-4 hrs all the oxygen present in the sewage get exhausted and it starts emitting offensive odours of hydrogen sulphide gas and other sulphur compounds produced by anaerobic micro organism

Temperature: The temp of waste water is higher than that of the water supply due to addition of warm water from industries & households. When it flows in closed circuit its temp rises further. The avg. temp of waste water in India is around 20°C which is quite close to ideal temp of biological activities. The change in temp affects the waste water in the following ways:

- (1) Extremely low temp affects adversely the efficiency of sedimentation
- (2) Bacterial activity ↑ with ↑ in temp upto 60°C and then falls. It affects the design of waste water treatment units and its efficiency
- (3) Solubility of gases in wastewater ↓ with ↑ in temp & consequent reduction in self purification of streams due to depletion of dissolved oxygen (DO)
- (4) Abnormally high temp can enhance the growth of undesirable water plants and waste water Analysis
fungus

Turbidity: it is a measure of light emitting properties of water and is used to indicate the quality of waste discharge wth colloidal matters. For more concentrated sewage, turbidity is higher can be measured by turbidimeter (turbid rod also used) (NTU-unit).

2. Chemical characters.



The total solids is defined as all the matters that remains as residue upon evaporation to 103-105°C. It is classified into 2, Fixed & volatile solid

Volatile solids is calculated in the following way

The dry residue obtained by evaporating the waste water @ 105°C & subjected to heating & ignition at 550°C in a muffle furnace about (15-20 min). The loss of wt due to this operation is the volatile solids

Assignment

1. Define sullage, storm water, sewage and night soil
2. Explain time of concentration

- Q. Determine the size of 0.15 sewer for a discharge of 700 lps running half full. Assume $f = 0.0001$, $n = 0.015$
4. Discuss the merits and demerits of separate & combined system of sewage
 5. Discuss the purpose served by an inverted siphon with the help of a neat sketch
 6. Explain physical characteristics of sewage

~~Chemical characteristics of sewage~~

Chemical characteristic of sewage

- | | |
|--|-------------------------------|
| 1. Total solids, suspended solids and settleable solid | 6. DO |
| 2. pH value | 7 BOD |
| 3. Nitrogen content | 8 COD |
| 4. chloride content | 9. sulphide, Sulphate, H_2S |
| 5. Fat, oil and grease | |

(b) pH value

Normally a fresh sewage is alkaline in nature as the time passes pH decreases due to anaerobic or nitritification process

$$pH = -\log[H]^+$$

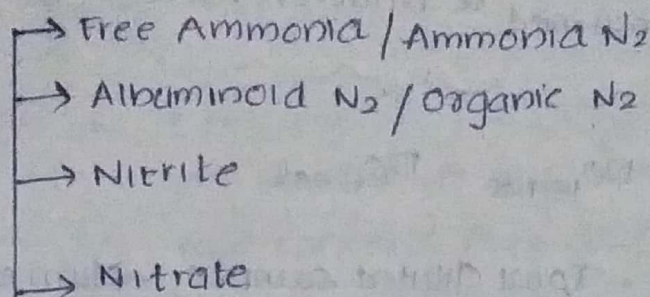
(c) chloride contents

Domestic sewage $\rightarrow 120 \text{ mg/l}$

Normal water / drinking water $\rightarrow 250 \text{ ppm}$

If the chloride content is high, it indicates the presence of industrial waste and also the infiltration of sea water. If the chloride content is high it indicates high strength of sewage.

(d) Nitrogen content



Free Ammonia: It indicates the very 1st stage of organic matter decomposition.

Albuminoid or organic N₂: It indicates the quantity of N₂ present in the sewage before the starting of organic matter decomposition.

org - Bio
Free - IS+
nitrite -> Partly
nitrate -> Fully

Nitrite: It indicates the presence of partially decomposed organic matter which indicates the treatment given is incomplete.

Nitrate: Indicates the presence of fully oxidised organic matter. It indicates well oxidised and treated sewage.

(e) Fat, oil and grease: It forms a scum on the top of sedimentation tank and also in the case of filtration it may clog the voids of filtering media.

(f) Sulphides, sulphates and H₂S gas: During aerobic decomposition sulphur compounds oxidise to sulphates and sulphides and during anaerobic decomposition sulphur compounds get reduced to sulphides with liberation of H₂S, CO₂ and Methane gases.

Aerobic: Sulphate → sulphate & sulphide
Anaerobic: Sulphate → sulphide (CH₄, CO₂, H₂)

(g) Dissolved oxygen (DO)

4 ppm (min) ⇒ For sustainability of aquatic life

(h) COD ⇒ Chemical Oxygen Demand

Measure of total organic matter includes biodegradable and non-biodegradable.

(i) BOD ⇒ Biochemical Oxygen Demand

Measure of biologically degradable organic matter.

$$BOD_5 = DO_{\text{consumed}} \times \text{Dilution Factor}$$

$$DO_{\text{consumed}} = DO_{\text{initial}} - DO_{\text{final}}$$

$$\text{Dilution Factor} = \frac{\text{Total diluted sample volume } V_{\text{diluted}}}{\text{Vol. of undiluted sewage sample } V_{\text{undiluted}}}$$

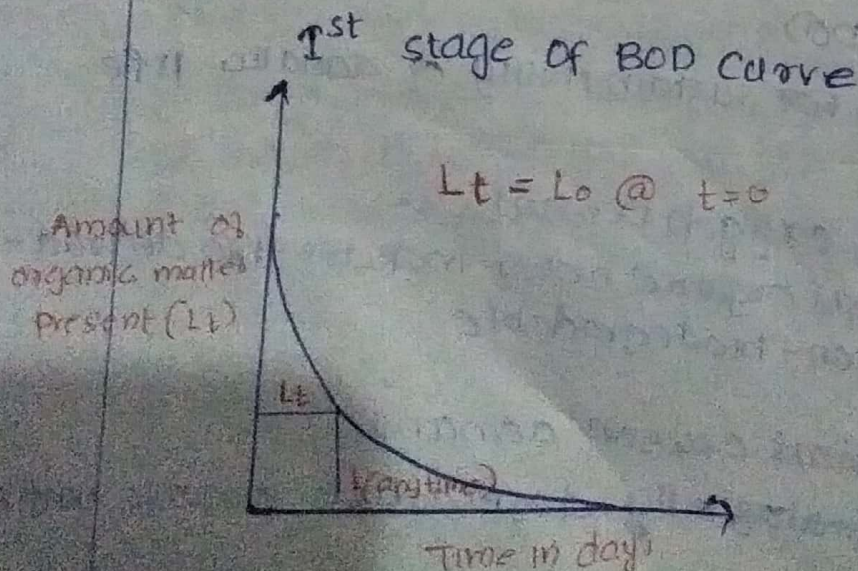
$$BOD_5 = 68\% BOD_{\text{total}} \Rightarrow 5 \text{ days incubation}$$

$$BOD_{10} = 90\% BOD_{\text{total}} \Rightarrow 10 \text{ days incubation}$$

BOD of water during 5 day @ 20°C is generally taken as standard demand. If oxygen supply is for more than 5 day the oxygen is consumed rapidly for 6-7 days then slows down until 20 days then it again accelerates for some time and again slows down. The BOD for 1st 20 days is taken as the carbonaceous demand.

The carbonaceous demand is also known as the 1st stage demand, or initial demand. After 20 days the demand is known as nitrogenous demand or 2nd stage demand.

The rate @ which BOD is satisfied @ any time depends on temp and also on the amount and the nature of organic matter present in sewage @ that time.



$$\frac{dL_t}{dt} = -kL_t \longrightarrow \textcircled{1}$$

where, L_t = Amount of 1st stage BOD remaining in sample at any time, t (mg/L)

k = rate constant (per day)

t = time (in days)

Integrating $\textcircled{1}$

$$\int_{L_0}^{L_t} \frac{dL_t}{L_t} = \int -k dt$$

$$\text{Log}_e L_t = -kt + C \quad (@ t=0, L_t=L_0)$$

$$\therefore \text{Log}_e L_0 = C$$

$$\Rightarrow \text{Log}_e L_t = -kt + \text{Log}_e L_0$$

$$\text{Log}_e \left(\frac{L_t}{L_0} \right) = -kt$$

$$2.3 \text{Log}_{10} \left(\frac{L_t}{L_0} \right) = -kt$$

$$\text{log}_{10} \left(\frac{L_t}{L_0} \right) - \frac{-kt}{2.3} = -0.43 kt = -k_D t$$

where $k_D = D_e \Rightarrow$ oxygenation constant

$$\text{log}_{10} \left(\frac{L_t}{L_0} \right) = -k_D t$$

$$\left(\frac{L_t}{L_0} \right) = L_0 10^{-k_D t}$$

$Y_t = L_0 - L_t$ (Quantity of organic matter oxidized during t days)

$$= L_0 - L_0 10^{-k_D t}$$

$$\boxed{Y_t = L_0 (1 - 10^{-k_D t})} \quad (\text{1st stage BOD reaction})$$

Ultimate BOD:

$$\text{when } t = \infty, Y_{\infty} = L_0(1 - 10^{-\infty}) = L_0(1 - \frac{1}{10^{\infty}})$$

$$Y_{\infty} = L_0$$

L_0 → Fixed value doesn't depend on temp

k_p → value determines speed of reactn

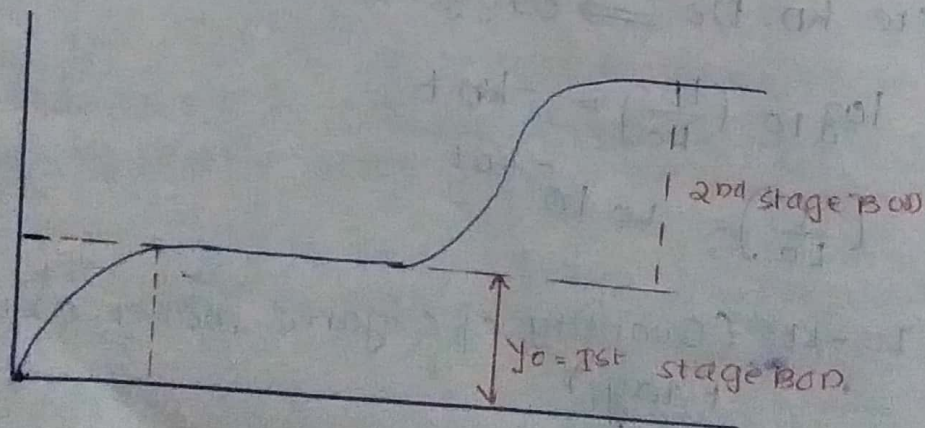
(deoxygenation constant)

$$k_p(T^{\circ}) = k_p(20^{\circ}) (1.047)^{T-20}$$

Generally $k_p = 0.05$ to 0.2 (per day)

Water type	k_p (per day)
Tap water	< 0.05
Surface water	$0.05 - 1$
Municipal waste water	$0.1 - 0.15$
Treated Sewage water	$0.05 - 0.1$

Combined BOD curve



Q1. A 25ml of raw sewage has been diluted to 250ml and DO concentration of dil sample @ the beginning of BOD test was 8 mg/l and 5 mg/l after 5 days incubation @ 20°. Find BOD of raw sewage

Ans

$$BOD = DO \times DF$$

$$\text{Dilution Factor } DF = \frac{250}{2.5} = 100$$

$$BOD = (8-5) \times 100 = 300 \text{ mg/l}$$

Given

$$D_{\text{initial}} = 8$$

$$D_{\text{final}} = 5$$

$$V_{\text{diluted}} = 250$$

$$V_{\text{undiluted}} = 2.5$$

$$D_0 = D_{\text{initial}} - D_{\text{final}}$$

$$8 - 5 = 3$$

$$BOD = D_{0_{\text{obs}}} \times DF$$

$$= D_{0_i} - D_{0_f} \times \frac{V_{\text{diluted}}}{V_{\text{undiluted}}}$$

$$(8-5) \times \frac{250}{2.5}$$

Q2. For the waste water sample the 5 day BOD @ 20°C is 200 mg/l and is 67% of the ultimate. What will be the 4 day BOD @ 30°C?

$$Y_t = L_0(1 - 10^{-kDt})$$

$$Y_t = Y_5 = 200 \text{ mg/l}, t = 5$$

$$Y_5 = 0.67 L_0$$

$$\Rightarrow 200 = 0.67 L_0$$

$$L_0 = \frac{200}{0.67} = 298.507$$

$$200 = 298.507 (1 - 10^{-kD_5})$$

$$= \frac{200}{298.507} - 1 = -10^{-kD_5}$$

$$= 10^{-kD_5} = 0.33$$

$$-kD_5 = -0.48$$

$$k_{D(20)} = 0.0963 \text{ per day}$$

$$k_{D(30)} = k_{D(20)} \theta^{T-20}$$

$$= 0.0963 \left(\frac{1.047}{1.056} \right)^{30-20}$$

$$= 0.166 \text{ per day}$$

∴ 4 day BOD @ 30°C

$$Y_4 = 298.507 (1 - 10^{(-0.166 \times 4)})$$

$$= 233.799 \text{ mg/l}$$

Q₃. During BOD test conducted on a 5% solution of waste of following observations were taken

* D_0 aerated water used for dilution = 3.6 mg/l

* D_0 original sample = 0.8 mg/l

* D_0 diluted sample after 5 day incubation = 0.7 mg/l

compute (i) 5 day BOD (ii) ultimate BOD

Assume decay constant @ the test temp as 0.12

Ans

100 — 5% sewage water
 — 95% aerated "

Diluted sample contain 5% sewage water and 95% aerated water

$$\therefore D_0 \text{ of test specimen} = \left(D_{0 \text{ wastewater}} \times \text{content} \right) + D_{0 \text{ aerated}} \times \text{content}$$

$$= 0.8 \times 0.05 + 3.6 \times 0.95$$

$$= 3.46 \text{ mg/l}$$

D_0 of incubated water after 5 days = 0.7 mg/l

$$\therefore D_0 \text{ consumed} = D_{0 \text{ initial}} - D_{0 \text{ final}} = 3.46 - 0.7 = 2.76 \text{ mg/l}$$

$$(i) \text{ BOD}_5 = D_0 \text{ consumed} \times DF = 2.76 \times 100/5 = 55.2 \text{ mg/l}$$

$$(ii) \text{ Ultimate BOD} = L_0 (1 - 10^{-kDt})$$

$$Y_5 = L_0 (1 - 10^{-kDt})$$

$$55.2 = L_0 (1 - 10^{-0.12 \times 5}) \Rightarrow L_0 = \underline{\underline{73.717 \text{ mg/l}}}$$

Q₄. The BOD of a sewage incubated for 1 day @ 30°C has been found to be 100 mg/l. what will be the 5 day 30°C to 20°C. Assume k @ 20°C as 0.12.

BOD $Y_1 = 100 \text{ mg/l}$ @ 30°C

$$k(30^\circ) = k(20^\circ) @ T-20$$

$$= 0.12 \times (1.056)^{30-20} = 0.207 \text{ per day}$$

$$Y_1 = L_0 (1 - 10^{-kDt})$$

$$Y_1(30^\circ) = L_0 (1 - 10^{-0.207 \times 1}) = 100$$

$$L_0 = 263.761 \text{ mg/l}$$

$$Y_5(20^\circ) = 263.761 (1 - 10^{-0.12 \times 5})$$

$$= 197.507 \text{ mg/l}$$

Population Equivalent

Industrial waste waters are generally compared with per-capita normal domestic waste waters so as to rationally charge the industries for the pollution caused by them.

$$\text{Population Equivalent} = \frac{\text{Std. 5 day BOD of industrial sewage}}{\text{Std 5 day BOD of domestic sewage}} \times \text{Per person per day}$$

↓
0.08 kg/person/day

Q. The avg sewage flow from a city 80 MLD. If the avg ^{5 day} BOD is 285 mg/l compute the total daily 5 day oxygen demand in kg and the population equivalent of sewage. Assume per capita BOD of sewage per day = 75 g

Ans.

$$\frac{\text{kg/day}}{\frac{\text{kg}}{\text{person/day}}} = \text{PERSON}$$

Quantity of sewage flowing per day = 80×10^6 l

Avg. 5 day BOD = 285 mg/l

Total 5 day BOD = $285 \times 80 \times 10^6$ mg/day

= 2.28×10^{10} mg/day

= 22800 kg/day

Population Equivalent = $\frac{\text{Std 5 day BOD of indu. sewage}}{\text{Std } \Rightarrow \text{ of domestic sewage}} \times \text{Per person/day}$

= $\frac{22800 \text{ kg/day}}{\frac{75}{1000} \text{ kg/person/day}}$

= 3,04,000

Note: population equivalent indicate the strength of industrial waste water for estimating the treatment required @ municipal sewage treatment plant and also helps in assessing the charges for the treatment from the industries

Relative Stability

Relative stability = $\frac{\text{Oxygen available in effluent}}{\text{Oxygen required to satisfy first stage BOD demand}}$

$$S = 100(1 - 0.794^{t_{20}})$$

$$S = 100(1 - 0.63^{t_{31}})$$

Q. If period of incubation is $10 \text{ days}^{t_{20}}$ @ 20°C in the relative conductivity test on sewage calculate the % of relative stability

Since 20°C use 1st equation

$$S = 100(1 - 0.794^{t_{20}})$$

$$= 100(1 - 0.794^{10})$$

$$= \underline{\underline{90.04\%}}$$